





East Sussex County Council Transport and Environment

Bexhill to Hastings Link Road G04 Powdermill Stream South Underbridge Approval in Principle

Doc. Ref: B1297000-PH2/1600.06a/0020

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Bexhill to Hastings Link Road

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Council Project No: Sussex County

B1297000

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G04 - Powdermill Stream South Underbridge - Approval in Principle

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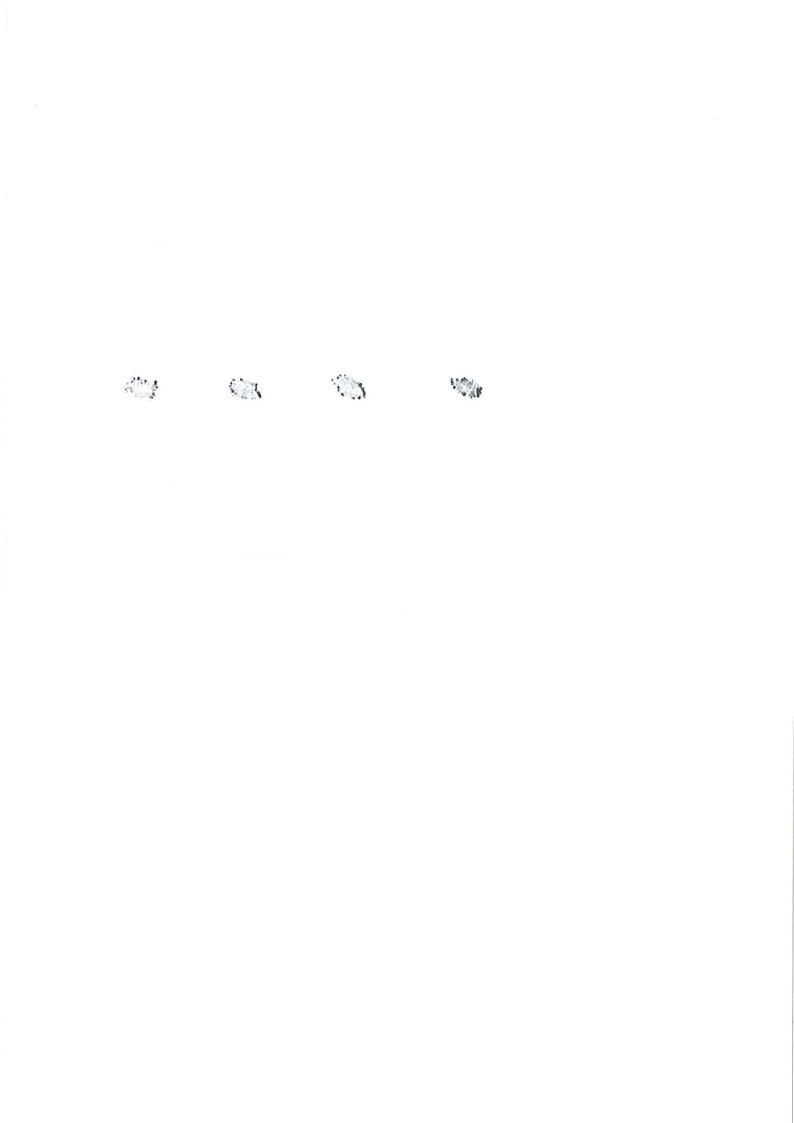
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Revision Summary

Client: Project: East Sussex County Council Bexhill to Hastings Link Road

Document Title:

G04 Powdermill Stream South Underbridge AIP

Transport and Environment

Job No: B1297000

REVISION / DATE	COMMENT
Rev 0 14/09/12	Amended to incorporate TAA comments raised on Phase 1 AIP ref. JB-B1297000-PH1/1600.06a/0020 (rev 0)
	Approach embankment ground treatment proposals added.
	Bored piles foundation changed to spread footing.
	Steel/timber bridleway parapet proposed.
	Steel deck waterproofing Departure added.
	Geotechnical information updated.

1. HIGHWAY DETAILS

1.1 Type of highway

Greenway - 3.65m wide shared equestrian, pedestrian, cyclist and Environment Agency maintenance access track with 0.6m wide margins either side.

1.2 Permitted traffic speed

Over: N/A Under: N/A

1.3 Existing restrictions

The structure spans a Main River for which the Environment Agency bears responsibility.

2. SITE DETAILS

2.1 Obstacles crossed

Powdermill Stream, an Environment Agency designated Main River with a square top of bank to top of bank width of approximately 10.0m.

3. PROPOSED STRUCTURE

3.1 Description of structure

Powdermill Stream South Underbridge is located at OS grid reference 576189.6E, 110614.7N. It is approximately 50m to the south of the Bexhill to Hasting Link Road Powdermill Stream underbridge. It carries the proposed Greenway over the Powdermill Stream.

The structure is a proprietary single span painted steel bridge supported on reinforced concrete abutments.

3.2 Structural type

Simply supported single-span main and secondary steel girders supporting stiffened steel decking plates and metal parapets. Main girders are located on bearings supported on reinforced concrete abutments.

The wingwalls are both cantilevered from the bankseats and free-standing gravity cantilever retaining walls with partially debonded dowel connections to mitigate differential settlement between structural elements.

Approach embankments and earthworks local to the structure are supported on controlled modulus column ground treatment to control differential settlement between earthworks, bridge abutments and free-standing wing walls. The ground treatment will be undertaken in advance of the structural foundation construction.

3.3 Foundation type

Reinforced concrete abutment founded on spread footings.

3.4 Span arrangements

Square span (between bearing centrelines): 17.2m Skew angle: 0.0 degrees

3.5 Articulation arrangements

Bearings will be elastomeric with longitudinal fixity provided on one abutment.

Expansion joints will consist of steel cover plates.

3.6 Types of road restraint systems

1.8m high bridleway parapets. Steel parapet posts with timber rails and timber infill to be designed for Class 3 post and rail loading and Type C infill loading to BS7818.

3.7 Proposed arrangements for maintenance and inspection

3.7.1 Traffic management

N/A

3.7.2 Access

A minimum 2.0m wide set back from top of bank will be provided in front of bearing shelves. A minimum 1.5m clear headroom will be provided from the maintenance area in front of abutments to the deck soffit between main girders. Bearing shelves will be a minimum of 600mm above the adjacent maintenance platform level.

Inspection and maintenance of the abutments and wing walls can be carried out at ground level or from a ladder or temporary scaffold for upper areas.

Foundations will not be visible or accessible for inspection.

The deck soffit and outer parapet faces can be accessed by scaffold/ladder from the river banks or from the track above using a small underbridge unit.

Bearings at abutments will be set on plinths as necessary to provide 300mm minimum clearance between the beam soffit and bearing shelves for inspection and maintenance purposes. Jacking points will be provided between main beams for bearing replacement. The bridge will be designed to carry its full design load during bearing replacement.

3.8.1 Materials and finishes

Concrete	Element		Limiting Exposure Class
C40/50	Substructure, buric Substructure, expo		DC1 XC3/4, XF3
Reinforcement		Grade B500B or grade:2005	B500C deformed bars to BS4449
			steel : Strength Grade 500, 4436 complying with BS
Structural steel	work	All structural steelwork or AdvanceS355J2 for	to BS EN 10025 Grade S355J2+N rolled sections.
Bolts		HSFG steel bolts to BS	S 4395 Part 1

Parapets 1.8m high bridleway parapet to BS 7818.

Parapet posts to be galvanised painted steel. Rails and

solid infill panels to be timber.

Backfill to abutments and

retaining walls

Class 6N/6P structural fill in accordance with DoT

Specification for Highway Works.

Concrete Finishes

Hidden and buried surfaces

F1/U1

Exposed faces of abutment and

wing walls

F6 grooved patterned profiled finish / U2

Protection

All accessible concrete surfaces greater than 150mm below finished ground level to receive waterproofing to below ground concrete surfaces in accordance with the SHW CI 2004.

All exposed concrete elements will receive anti-graffiti coating.

All structural steelwork shall be painted with an approved Type II (Inland, Difficult Access) paint system with a maintenance period of 20 years in accordance with DoT Specification for Highway Works.

Deck surfaces shall be coated with a suitable proprietary non-slip surfacing.

Parapet posts shall be galvanised steel painted with an approved Type IV paint system with a maintenance period of 20 years in accordance with the SHW.

Holly Green 14C39 Colour to BS 4800:1989 is proposed for all painted steelwork.

3.8.2 Sustainability issues

The materials and protective measures proposed are intended to maximise the durability of the structure and to minimise the requirement for future maintenance.

3.9 Risks and hazards considered

Standard construction methods are anticipated along with normally associated risks and hazards. The risks and hazards associated with the construction activities relating to these works will be identified by the appropriate method statements and safe working practices, to be completed prior to any construction taking place.

Risks associated with working at height and over water will be limited by maximising the amount of prefabrication of steelwork elements off-site.

3.10 Estimated cost of proposed structure together with other structural forms considered and the reasons for their rejection including comparative whole-life costs with dates of estimates.

The relative advantages, disadvantages and costs of various structural forms are discussed and appraised in Owen Williams reports No. 262701/012 'BHLR Structures Options Report' and No. 262701/060 'BHLR Structures Options Report – Addendum'.

3.11 Proposed arrangements for construction

3.11.1 Traffic management

N/A

3.11.2 Service diversions

N/A

3.11.3 Interface with existing structures

N/A

DESIGN CRITERIA

4.1 Live loading, Headroom

4.1.1 Loading relating to normal traffic under AW regulations and C&U regulations

3t Assessment Live Loading (ALL) in accordance with BD21/01, including wheel and axle loading, assuming Low Traffic Flow and Good Road Surface category.

4.1.2 Loading relating to General Order traffic under STGO regulations

Not required.

4.1.3 Footway or footbridge live loading

Foot/cycle track loading will be in accordance with BD 37/01 CI 6.5.1 with the width of pedestrian area considered as the full width of deck between parapets ie 4.85m. In accordance with CI 6.5.1 the pedestrian load intensity will be reduced by 15% in the 2m-3m width and 30% in the 3m-4.85m range. The applied load will be taken as the average intensity.

4.1.4 Loading relating to Special Order Traffic, provision for exceptional abnormal loads or indivisible loads, including location of vehicle track on deck cross-section

Not required.

4.1.5 Any special loading not covered above

EA maintenance access vehicle – 24t tracked excavator (based on JCB JZ 255). Load factors to be as per BD37/01 HA loading. Impact factor = 1.2 due to extreme low speed.

Construction loading from 6t dumper (W1 = 7.5t, W2 = 3.0t, A1 = 2.4m) to be applied in accordance with BD21/01 Annex D.

Approach embankments founded on soft ground will be subjected to advance works ground treatment ie controlled modulus column installation, in advance of structural foundation construction.

4.1.6 Heavy or high load route requirements and arrangements being made to preserve the route, including any provision for future heavier loads or future widening.

Not required.

4.1.7 Minimum headroom provided

From east side platform:

2.72m to soffit of discrete main steel girders.

3.27m to soffit of deck between discrete girders.

From west side platform:

- 4.84m to soffit of discrete main steel girders.
- 5.40m to soffit of deck between discrete girders.

Minimum headroom from 100yr flood level+20%:

- 5.67m to soffit of discrete main steel girders.
- . 6.03m to soffit of deck between discrete girders.

Minimum headroom required:

- 1.05m to soffit of discrete main steel girders.
- 1.5m to soffit of deck between discrete girders.
- 0.6m free-board above 100yr flood level+20%

4.1.8 Authorities consulted and any special conditions required

Authority Consulted Special Requirement

Environment Agency A minimum 2m margin on each bank

and the soffit level to be set a minimum 600mm above the

predicted 1 in 100 year flood (+20%)

level.

24t tracked excavator access requirement with minimum width of

12ft.

British Horse Society Non-slip deck surface.

Planning Condition number 5. Bridge abutments are to be set back 2m from top of waterway channel banks to facilitate green corridor and soft

bank solution.

1.5m minimum maintenance headroom to underside of structure between beams. 0.9m minimum maintenance headroom to underside

of discrete beams.

4.2 List of relevant documents from the TAS

See Appendix A

4.2.1 Additional relevant standards

BS 8500; Part 1; 2006 Concrete; Complementary British Standard to BS EN

206-1; Method of specifying and guidance for the

specifier.

BS 8500; Part 2; 2006 Concrete; Complementary British Standard to BS EN

206-1; Specification for constituent materials and

concrete.

CHE Memorandum 227/08

The Impregnation of Reinforced and Prestressed Concrete Highways Structures Using Hydrophobic Pore Lining Impregnants.

4.3 Proposed Departures from Standards given in 4.2 and 4.2.1

Implementation of CHE Memorandum 227/08 – Deletion of requirement for impregnation with hydrophobic pore lining Impregnants.

Modified longitudinal loading on Greenway structures – Deletion of BD37 requirements and provision for reduced alternative longitudinal load.

Application of combined waterproofing and surfacing to steel bridge decks – Use of proprietary anti slip coating system.

Refer to Appendix E.

4.4 Proposed methods for dealing with aspects not covered by Standards in 4.2 and 4.2.1 None.

5. STRUCTURAL ANALYSIS

5.1 Methods of analysis proposed for superstructure, substructure and foundations

A static analysis approach will be used to calculate design loadings on superstructure, substructure and foundations.

The main longitudinal girders and cross-members will be analysed manually as simply supported line beams.

The stiffened steel decking will be analysed manually.

The substructure and foundation will be analysed by simple hand calculations.

5.2 Description and diagram of idealised structure to be used for analysis.

See Appendix D

5.3 Assumptions intended for calculation of structural element stiffness

Element stiffness for steel members will be determined in accordance with BS 5400 Part 3:2000.

Element stiffness for concrete members will be derived in accordance with BS 5400 Part 4:1990 Clause 4.4, using full elastic uncracked member cross-sections ignoring the presence of reinforcement.

5.4 Proposed earth pressure coefficients (k_a , k_0 , or k_p) to be used in the design of earth retaining elements

For the analysis of the abutment and wing walls, k_a will be used for stability calculations and k_0 for structural element design. Soil parameters will be determined as a detailed design activity.

A representative peak angle of friction of 35° will be used for 6N/6P granular backfill, for which $k_a = 0.27$, $k_0 = 0.43$, and $k_p = 3.69$.

Back of wall friction will not be considered.

6. GEOTECHNICAL CONDITIONS

6.1 Acceptance of recommendations of Section 8 of the Geotechnical Report to be used in the design and reasons for any proposed changes.

Section 2 of the Geotechnical Report has not been completed at this stage.

6.2 Geotechnical Report Highway Structure Summary Information (Form C)

A draft Geotechnical Report Highway Structure Summary sheet based on the information available in Part 1 of the Geotechnical Report is attached in Appendix C. A full Geotechnical Report Highway Structure Summary sheet and extracts from the completed Geotechnical Report Section 2 will be produced following development of the Geotechnical Report.

6.3 Differential settlement to be allowed for in the design of the structure.

The structure is founded on spread footings on the firm to very stiff Ashdown Formation. A maximum differential settlement of 25mm between abutments will be considered.

6.4 If the Geotechnical Report is not yet available, state when the results are expected and list the sources of information used to justify the preliminary choice of foundations.

The preliminary choice of foundation is discussed in the draft Geotechnical Report Highway Structure Summary sheet contained in Appendix C. Part 2 of the Geotechnical Report, including Section 2 Highway Structures, is expected to be completed in Phase 2 of the project.

7. CHECKING

7.1 Proposed category of structure

Category 1 in accordance with BD2/05

7.2 If Category 3, name of proposed Independent checkers.

N/A

7.3 Erection proposals or temporary works for which an independent check will be required, listing parts of the structure affected with reasons for recommending an independent check.

N/A

8. DRAWINGS AND DOCUMENTS

8.1 List of drawings and documents accompanying the submission.

Appendix A List of relevant documents from TAS dated February 2009

Appendix B Drawing No Title

B1297000-PH2/1600.01A/9181 Powdermill Stream South Underbridge

Rev 0 (G04) General Arrangement

Appendix C Geotechnical Information

Appendix D Idealised Structure

Appendix E Departures from Standards

Doc. Ref: B1297000-PH2/1600.06A/0020 Rev 0



9 THE ABOVE IS SUBMITTED FOR ACCEPTANCE

9.1 Submission by designer

Signed



Name: P. Blackie

Position: Structures team leader, Jacobs

Engineering Qualifications: BEng(Hons), CEng MICE

Date: 14/09/12

9.2 Endorsement by contractor

Signed



Name: S. LAPTHORN

Engineering Qualifications: MEng(Hons) Cong MICE.

Position: Design Coordinator

Hochtief Vinci Joint Venture

Date: 20/09/12

10. THE ABOVE IS REJECTED/AGREED SUBJECT TO THE AMENDMENTS AND CONDITIONS SHOWN BELOW.

1

Reviewed:

Name:

Engineering qualifications:

Date:

Signed:

Name:

Engineering qualifications:

TAA

Date:





Appendix A List of Relevant Documents

Schedule of Design Documents Relating to Highway Bridges and Structures; February 2009

British Standards

BS 5268; Part 2; 2002	Structural Use of Timber			
BS 5400	Steel concrete and composite bridges			
Part 1; 1988	General Statement (see BD 15)			
Part 2; 1978	Specification for loads (see BD 37)			
Part 3; 2000	CP for design of steel bridges (see BD 13)			
Part 4; 1990	CP for design of concrete bridges (see BD 24)			
Part 5; 1979	CP for design of composite bridges (see BD 16)			
Part 9; 1983	Bridge bearings (see BD 20)			
Part 10; 1980	CP for fatigue (see BD-9)			
BS 5628; Part 1; 1992	Unreinforced-Masonry			
BS 5930; 1999	Site Investigations			
BS 6031; 1981	Earthworks			
BS 8002; 1994	Earth retaining structures			
BS 8004; 1986	Foundations			
BS 8118; 1991	The structural use of aluminium			
BS EN 1317-1-1998; Road Restraint Systems – Part 1	Terminology and general criteria for test methods			
BS EN 1317-2-1998; Road Restraint Systems – Part 2	Performance classes, impact test acceptance criteria and test methods for safety barriers			
BS EN 1317-3-2000; Road Restraint Systems Part 3	Performance classes, impact test acceptance criteria and test methods for crash-cushions			
DD ENV 1317-4-2002; Road Restraint Systems - Part 4	Terminals and transitions			
BS-EN 14388; 2005	Road traffic noise reducing devices - Specification			

Miscellaneous

Circular Roads No 61/72 - Routes for heavy and high abnormal loads.

Railway-Group-Approved Code of Practice GC/RC5510: Recommendations for the Design of Bridges (2000) (for full list of other Network Rail Standards, refer to RSSB, Railway-Safety and Standards Board)

Simplified Tables of External Loads on Buried Pipelines (1986)

Traffic Management Act 2004

The Manual of Contract Documents for Highway Works (MCDHW)

Volume 1: Specification for Highway Works 1998, including amendments to May 2009

Volume 2:

Notes for Guidance on the Specification for Highway Works 1998, including amendments to

May 2009

Volume 3:

Highway Construction Details 1991, including amendments to November 2008

The Design Manual for Roads and Bridges (DMRB)

Bridges and Structures (BA Series)

Reproduced on following pages

Bridges and Structures (BD Series)

Reproduced on following pages

Bridges and Structures, Technical Memoranda (BE Series)

Reproduced on following pages

Traffic Engineering and Control, Standards (TD Series)

TD 9/93

Road layout and geometry. Highway link design

TD 19/06

Requirement of Road Restraint Systems & correction No. 1

TD 27/05

Cross Sections and headroom

TD-36/93

Subways for pedestrians and cyclists, layout and dimensions

Highways, Advice Notes (HA Series)

HA 59/92

Mitigating Against Effects on Badgers

HA-80/99

Nature Conservation Advice in Relation to Bats

HA-84/01 (1)

Nature Conservation and Biodiversity

HA 97/01

Nature Conservation Management Advice in Relation to Dormice

HA-98/01

Nature Conservation Management Advice in Relation to Amphibians

Highways, Standards (HD Series)

HD 22/08

Managing Geotechnical Risk

	ADVICE NOTES - BRIDGES AND STRUCTURES (BA SERIES)
BA-9/81	The Use of BS-5400: Part 10: 1980. Code of Practice for Fatigue Amendment No. 1
BA-16/97	The Assessment of Highway Bridges and Structures. Amendment No. 1 Amendment No.2
BA 19/85	The Use of BS 5400: Part 3: 1982
BA 24/87	Early Thermal Cracking of Concrete Amendment No. 1
BA 26/94	Expansion Joints for Use in Highway Bridge Decks
BA 28/92	Evaluation of Maintenance Costs in Comparing Alternative Designs for Highway Structures
BA-30/94	Strengthening of Concrete Highway Structures Using Externally Bonded Plates
BA 34/90	Technical Requirements for the Assessment and Strengthening Programme for Highway Structures
BA-35/90	Inspection and Repair of Concrete Highway Structures
BA-36/90	The Use of Permanent Formwork
BA 37/92	Priority Ranking of Existing Parapets
BA 38/93	Assessment of the Fatigue Life of Corroded or Damaged Reinforcing Bars
BA 39/93	Assessment of Reinforced Concrete Half-joints
BA-40/93	Tack Welding of Reinforcing Bars
BA 41/98	The Design and Appearance of Bridges
BA-42/96	The Design of Integral Bridges [Incorporating Amendment No. 1 dated May 2003]
BA-43/94	Strengthening, Repair and Menitoring of Post-tensioned Concrete Bridge Decks
BA-44/96	Assessment of Concrete Highway Bridge and Structures
BA 47/99	Waterproofing and Surfacing Concrete Bridge Decks
BA 50/93	Post-tensioned Concrete Bridges: Planning, Organisation and Methods for Carrying Out Special Inspections
BA 51/95	The Assessment of Concrete Structures Affected by Steel Corrosion
BA 52/94	The Assessment of Concrete Highway Structures Affected by Alkali Silica Reaction
BA 53/94	Bracing Systems and the Use of U-Frames in Steel Highway Bridges
BA 54/94	Load Testing for Bridge Assessment
BA 55/06	The Assessment of Bridge Substructures and Foundations, Retaining Walls and Buried Structures
BA 56/96	The Assessment of Steel Highway Bridges and Structures
BA 57/01	Design for Durability
BA 58/94	Design of Bridges and Concrete Structures with External Unbonded Prestressing
BA 59/94	Design of Highway Bridges for Hydraulic Action
BA 61/96	The Assessment of Composite Highway Bridges
BA 67/96	Enclosure of Bridges
BA 68/97	Crib-Retaining-Walls
BA-72/03	Maintenance of Road Tunnels

	ADVICE NOTES - BRIDGES AND STRUCTURES (BA SERIES)
D1 74/00	
BA 74/06	Assessment of Scour at Highway Bridges
BA-80/99	Use of Rock-Bolts
BA 82/00	Formation of Continuity Joints in Bridge Decks
BA-83/02	Cathodic Protection for Use in Reinforced Concrete Highway Structures
BA 84/02	Use of Stainless Steel Reinforcement in Highway Structures
BA 85/04	Coatings for Concrete Highway Structures & Ancillary Structures
BA-86//06	Advice Notes on the Non-Destructive Testing of Highway Structures
BA-87//04	Management of Corrugated Steel Buried Structures
BA-88//04	Management of Buried Concrete Box Structures
BA 92/07	The Use of Recycled Concrete Aggregates in Structural Concrete
BA 93/09	Structural Assessment of Bridges with Deck Hinges
	STANDARDS - BRIDGES AND STRUCTURES (BD SERIES)
BD 2/05	Technical Approval of Highway Structures
BD-7/01	Weathering Steel for Highway Structures
BD 9/81	Implementation of BS 5400: Part 10: 1980. Code of Practice for Fatigue
BD 10/97	Design of Highway Structures in Areas of Mining Subsidence
BD-12/01	Design of Corrugated Steel Buried Structures with Spans Greater than 0.9 Metres and up to 8.0 Metres
BD 13/06	Design of Steel Bridges. Use of BS 5400: Part 3: 2000
BD 15/92	General Principles for the Design and Construction of Bridges. Use of BS 5400: Part 1: 1988
BD 16/82	Design of Composite Bridges. Use of BS 5400: Part 5: 1979 Amendment No. 1
BD 20/92	Bridge Bearings. Use of BS 5400: Part 9: 1983
BD 21/01	The Assessment of Highway Bridges and Structures
BD 24/92	Design of Concrete Bridges. Use of BS 5400: Part 4: 1990
BD 27/86	Materials for the Repair of Concrete Highway Structures
BD 28/87	Early Thermal Cracking of Concrete Amendment No. 1
BD 29/04	Design Criteria for Footbridges
BD 30/87	Backfilled Retaining Walls and Bridge Abutments
BD 31/01	The Design of Buried Concrete Box and Portal Frame Structures
BD 33/94	Expansion Joints for Use in Highway Bridge Decks
BD-34/90	Technical Requirements for the Assessment and Strengthening Programme for Highway Structures
BD 35/06	Quality Assurance Scheme for Paints and Similar Protective Coatings
BD 36/92	Evaluation of Maintenance Costs in Comparing Alternative Designs for Highway Structures
BD 37/01	Loads for Highway Bridges

	STANDARDS - BRIDGES AND STRUCTURES (BD SERIES)
BD-41/97	Reinforced Clay Brickwork Retaining Walls of Pocket Type and Grouted Cavity type Construction Use of BS 5628: Part 2: 1995
BD-42/00	Design of Embedded Retaining Walls and Bridge Abutments
BD-43/03	The Impregnation of Reinforced and Prestressed Concrete Highway Structures using Hydrophobic Pore-Lining Impregnants
BD-44/95	The Assessment of Concrete Highway Bridges and Structures
BD 45/93	Identification Marking of Highway Structures
BD 46/92	Technical Requirements for the Assessment and Strengthening Programme for Highway Structures [Stage 2 - Modern Short Span Bridges]
BD 47/99	Waterproofing and Surfacing of Concrete Bridge Decks
BD 48/93	The Assessment and Strengthening of Highway Bridge Supports
BD 49/01	Design-Rules for Aerodynamic Effects on Bridges
BD-50/92	Technical Requirements for the Assessment and Strengthening Programme for Highway Structures Stage 3 — Long Span Bridges
BD 51/98	Portal and Cantilever-Signs/Signal Gantries
BD 53/95	Inspection and Records for Road Tunnels
BD-54/93	Post-tensioned Concrete Bridges, Prioritisation of Special Inspections
BD-56/96	The Assessment of Steel Highway Bridges and Structures
BD 57/01	Design for Durability
BD 58/94	The Design of Concrete Highway Bridges and Structures with External and Unbonded Prestressing Design of Highway Bridges for Vehicle Collision Loads
BD-60/04	Design of Highway Bridges for Vehicle Collision Loads
BD 61/96	The Assessment of Composite Highway Bridges
BD 62/07	As Built, Operational and Maintenance Records for Highway Structures
BD 63/07	Inspection of Highway Structures
BD 65/97	Design-Criteria for Collision Protector Beams
BD 67/96	Enclosure of Bridges
BD 68/97	Crib Retaining Walls
BD 70/03	Strengthened/Reinforced Soils and Other Fills for Retaining Walls and Bridge Abutments Use of BS8006: 1995, incorporating Amendment No. 1 (Issue 2 March 1999)
BD 74/00	Foundations
BD 78/99	Design of Road Tunnels
BD-79/06	The Management of sub Standard Highway Structures
BD 81/02	Use of Compressive Membrane Action in Bridge Decks
BD-82/00	Design of Buried Rigid Pipes
BD-84/02	Strengthening of Concrete Bridge-Supports Vehicle Impact Using Fibre Reinferced Polymers
BD-85/08	Strengthening Highway Structures Using Externally Bonded Fibre Reinforced Polymer
BD-86/07	The Assessment of Highway Bridges and Structures For The Effects of Special Types General Order (STGO) and Special Order (SO) Vehicles
BD 87/05	Maintenance Painting of Steelwork

	STANDARDS - BRIDGES AND STRUCTURES (BD SERIES)
BD-89/03	The Conservation of Highway Structures
BD-90/05	Design of FRP Bridges and Highway-Structures
BD 91/04	Unreinforced Masonry Arch Bridges
BD-94/07	Design of Minor Structures
BD-95/07	Treatment of Existing Structures on Highway widening Schemes
	TECHNICAL MEMORANDA - BRIDGES (BE SERIES)
BE-13	Fatigue Risk in Bailey Bridges
BE-23	Shear Key Decks Amendment No. 1 to Annex
BE-5/75	Rules for the Design and Use of Freyssinet Concrete Hinges in Highway Structures
BE 7/04	Departmental Standard (Interim)
	Motorway Sign/Signal Gantries
	INTERIM ADVICE NOTES (IAN)
IAN 117/08 Rev 1	Certification of combined kerb and drainage products
IAN-116/08	Nature conservation advice in relation to bats
IAN 104/07	The Anchorage of Reinforcement and Fixings in Hardened Concrete
IAN-97/07	Assessment and upgrading of existing parapets
IAN-96/07r1	Guidance on implementing Results of Research on Bridge Deck Waterproofing
IAN 95/07	Revised Guidance Regarding the Use of BS8500(2006) For the Design and Construction of Structures Using Concrete
IAN-91/07	Interim Advice on the identification of "Particularly at Risk" supports
IAN 70/06	Implementation of New Reinforcement Standards
IAN 69/05	Design for Maintenance
IAN 48/03	Measures To Minimise The Risk of Sulphate Attack (Including Thaumasite) - New Construction and Structures Under Construction
IAN 47/02	Post Tensioned Grouted Duct concrete Bridges
IAN 41/02	European Cement Standards
IAN 05/96	BD 24/92 The Design of Concrete Highway Bridges and Structures. Use of BS 5400: Part 4:1990

BD-44/95 The Assessment of Concrete Highway Bridges and Structures

BA 50/93 Post Tensioned concrete Bridges

IAN-04/96 IAN-03/96

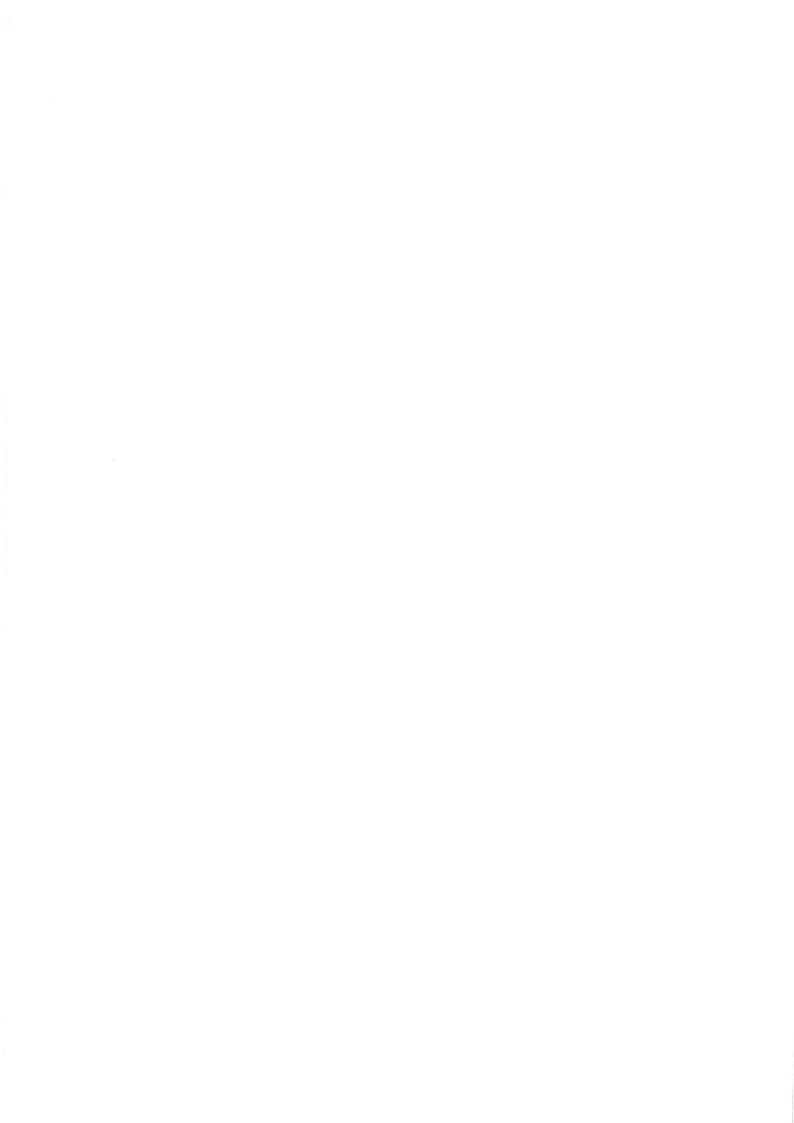
Appendix B Drawings

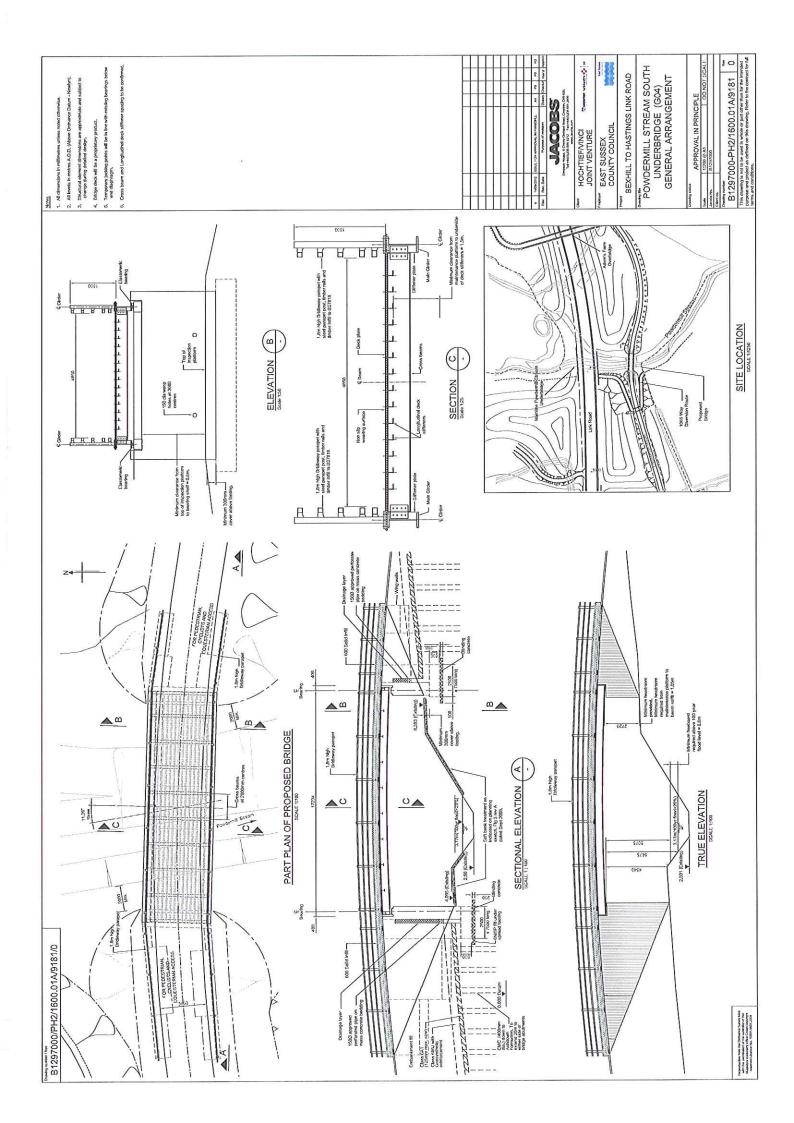
Drawing No

Title

B1297000-PH2/1600.01A/9181

Powdermill Stream South Underbridge (G04) General Arrangement







Appendix C Geotechnical Information

Doc. Ref: B1297000-PH2/1600.06A/0020 Rev 0

BEXHILL TO HASTINGS LINK ROAD

GEOTECHICAL SUMMARY INFORMATION

	OLO I LOTHIO / LL COMMIN (1/1 I W CYMIN/ 1/10)				
STRUCTURE NAME	RUCTURE NAME CHAINAGE and OS Grid Reference				
G04 - Powdermill Stream South UB	Ch 4015 OS: 576189.6E, 110614.7N				
Rev: 1	DESIGN LIFE: 120	/ears			
SOILS/GEOLOGY	RELEVANT EXPLO	RATORY	HOLES:		
	BH16 (May Gurney,	, 2006)			
	BH194 (URS Investi	gation, 20	09)		
Strata		Typical	depths		
West					
Topsoil/Made Ground		3.7 to 2.	7m OD		
Ashdown Formation – interbed clay	lded sands, silts and	below 2.7m OD			
<u>East</u>			4m OD		
Topsoil		6.7 to 6.4m OD below 6.4m OD			
Ashdown Formation – interbed clay	lded sands, silts and	below 6.	4m OD		
PREVIOUS GROUND Agricultural land HISTORY					
CONTAMINATED GROUND RISK ASSESSMENT No REQUIRED					
GROUNDWATER					
Groundwater was encountered in BH16 initially at 2.62m OD (4.1m bgl) within Ashdown Formation and rose to a level of 4.12m OD (2.6m bgl) in 20 minutes A second groundwater strike encountered confined groundwater within Ashdown Formation at -2.78m OD (9.7m bgl) and rose to a level of 3.47m OD (3.25m bgl) in 20 minutes. Groundwater monitoring carried out between April 2006 and March 2010 indicates a level of up to 1.3m bgl.					
Allowing for seasonal fluctuations, the preliminary groundwater level is assumed to be 3.4m OD (0.3m bgl based on the west side)					

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Spread Footings

EARTH PRESSURE VALUE K₀* K_a* Kp*

Refer to Section 5.4 of the AIP.

TYPE OF

FOUNDATION

BEARING CAPACITY	Spread Fo	Spread Footings				
Structure Element	Founding Stratum	Found Level (m AC		Footing Size	Allowable Bearing Pressure (kN/m²)	
West Abutment	Ashdown Formation	2.495		7.5m x 5m	200kN/m ²	
East Abutment	Ashdown Formation	4.89		7.5m x 3m	200 kN/m ²	
PILE DESIGN:	N/A			L		
Structure Element	Founding Stratum	Toe Level (mAOD)	Pile dia (m)	Pile length (m)	Pile working Load (kN)	
Note: Pile length	s and toe le	vels are a	pproxir	nate – pile cap	elevations to be confirmed.	
Pile type:						
Criteria for selec	ting pile toe	level:				
Allowance for ne	gative skin	friction wit	hin des	sign:		
SETTLEMENT						
Differential settle	ement to be	allowed fo	r betwe	een adjacent su	ipports: 25mm	
Differential settlement to be allowed between structure and approach embankment: 20mm (within 10 metres of the interface between structures and approach embankments)						
CHEMICAL ANALYSIS						
Buried Concrete classification:						
The results of chemical tests on soil samples taken within the rural areas indicate pH values ranging between 3.8 to 9.4 and sulfates (2:1 Water Extract) values of between 10 to 900mg/l. The recommended Design Sulfate and Concrete Classification based on BRE Special Digest 1 (2005) are DS-2 and AC-3z respectively.						
NOTES						

Approval in Principle

- 1. The ground sequence at the site is Topsoil/Made Ground and Ashdown Formation. Made Ground is not recommended as a bearing stratum due to its unpredictable bearing behaviour and poses a risk of differential settlement taking place. The maximum thickness of Topsoil/Made Ground is 1m.
- 2. Based on the formation levels (2.495m OD West Abutment & 4.89m OD East Abutment) the structure will likely be founded on the firm to very stiff Ashdown Formation.
- 3. The behaviour of the groundwater indicates likely presence of confined aquifer. This should be considered during construction.

MAY	GURNEY	/			Site Bexhill to Hastings Link Road				hole ber 16			
Boring Met Cable Percu	hod	Casing 15	Diamete 0mm cas	r ed to 7.00m	Ground	Leve 6.72		Client East Sussex County Council				ber 085
		Location 57	Dates 08 09	3/03/2 3/03/2	006- 006	Engineer Owen Williams		S	hee 1/			
Depth (m)	Sample / Tests	Casing Depth Cim Pield Reco		Field Records	Level (mOD)	D (Thi	epth (m) ckness)	Description	Legeno	Mater Insti		str
0.30-0.80	B1				6.42	بلىسىنار	(0.30) 0.30 (0.50)	TOPSOIL. Firm brown slightly gravelly sandy CLAY. Gravel is angular to subrounded fine and medium flint. Firm mottled orange brown and light brown sand				
0.80-1.20	B2				5.92		0.80	SILT / CLAY. Very stiff mottled orange brown, light brown and light grey sandy CLAY with rare black organic pockets to 3.30m	×			
1.50	U1			32 blows								
1.95 2.00-2.50	D1 B3											
2.50-2.95 2.50	SPT N=19 D2	1.30		2,2/3,6,5,5			(3.70)			V 1	/	100 mg
3.50 3.50-3.95	U2 D3			50 blows								
4.00-4.50 4.10	B4 W1			FAST(1) at 4.10m, rose to 2.55m in						∇1		
4.50 4.50-4.87	D4 SPT 50/220	2.90	2.90	20 mins, sealed at 7.00m. 7,10/14,20,16	2.22		4.50	Very sliff mottled brown and gey SILT	* * * * * * * * * * * * * * * * * * *			
5.50	U3			100 blows		انانانانانانا	(2.50)		< x x x x x x x x x x x x x x x x x x x			
6.00-6.50	B5								* * * * * * * * * * * * * * * * * * *			
6.50-6.95 6.50-6.95	SPT N=45 D5	5.90	4.80	14,11/10,8,11,16	-0.28		7.00		<pre></pre>			
					0.20		7.00	Very stiff light grey CLAY with pockets of orange and brown fine sand.				
7.50 7.50	D6 U4		9	50 blows								
8.00-8.50	B6											
8.50-8.95 8.50-8.95	SPT N=50 D7	7.00		6,7/10,12,12,16								
9.50 9.50 9.70	D8 U5 W2			FAST(2) at 9.70m, rose to 3.30m in 100 blows 20 mins.						∇ 2		
Remarks Hand exavat 50mm diame PID reading	ted pit to 1.20m eter standpipe installe adjacent to BH at 0.3	ed with rai	sed cove	r protected with fence	i				Scale (approx)	L _G	ogge y	∌d
Hand exavated pit to 1.20m 50mm diameter standpipe installed with raised cover protected with fence PID reading adjacent to BH at 0.30m bgl = 0.4ppm Water added from 1.50m to 4.10m. Water added from 7.00m to 9.70m.										1:50 JE/AK Figure No. \$11085 BH16		

MAYC	GURNEY						Site Bexhill to Hastings Link Road		Νι	orehole umber 3H16
Boring Method Cable Percussion		Casing 15	Diamete Omm cas	r ed to 7.00m	Ground	Level (mOD) 6.72	Client East Sussex County Council	1.000,000	ob umber SI1085	
	Locatio 57		B E 110629.515 N	Dates 08	3/03/2006- 9/03/2006	Engineer Owen Williams		Sł	heet 2/3	
Depth (m)	th) Sample / Tests		Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description Legend			Instr
10.00-10.50	B8			ı		E E		: <u></u> :		
10.50-10.95 10.50-10.95	SPT N=53 D9	7.00	3.30	12,13/16,14,10,13						
11.50 11.50	D10 U6			100 blows						
12.50-12.80 12.50	SPT 50/150 D11	12.50	3.70	11,14/22,28		(9.00)				
13.50 13.50	D12 U7			100 blows		-				
13.95	D13					Mara Mara				
14.50-14.82 14.50 14.50-15.00	SPT 50/170 D14 B9	14.50	3.90	15,10/20,19,11		ահատահա				
15.50 15.50 15.50-15.95	D15 U8 B10			100 blows	-9.28	16.00				
16.50-16.50 16.50-16.95 16.50 16.50-16.95	SPT N=50 D16 D17	16.30	4.10	6,9/9,10,14,17	-9.20	(2.00)	Weak SILTSTONE and brown COAL recovered as a grey and black sandy SILT. Gravel is angular to subangular fine to medium brown coal	(XXXXXX (XXXXXX (XXXXXX		
17.50 17.50	D18 U9			150 blows				<pre></pre>		
18.00-18.50	B12				-11.28	18.00	Weak SILTSTONE recovered as a slightly sandy SILT with occasional bands of coal.	******* ******* *******		
18.50-18.70 18.50	SPT 25*/80 50/115 D19	18.40	4.50	20,5/28,22		(2.15)				
19.50 19.50 20.00-20.14	D20 U10 50/70 SPT 25*/70	20.00	4.80	150 blows 25/50		المسلماما		<pre></pre>		
Remarks		•						Scale (approx)	Lo	ogged y
								1:50		E/AK
								Figure N SI108		H16

MAYC	GURNEY	/		Site Bexhill to Hastings Link Road	Borehole Number BH16				
Boring Meth Cable Percu	nod	Casing Dia 150m	ameter m cased to 7.00m	Ground	Level (mOD) 6.72	Client East Sussex County Council	N	ob umber SI1085	
		Location 57620	01.473 E 110629.515 N	Dates 08	8/03/2006- 9/03/2006	Englneer Owen Williams	SI	heet 3/3	
Depth (m)	Sample / Tests	Casing W Depth D (m)	/ater epth Field Records (m)	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water	Instr
20.00-20.15	D21			-13.43	(2.15) 20.15	Complete at 20.15m			
Remarks							Scale (approx) 1:50 Figure I	ار. اد	ogged y JE/AK

Contract No: 49325727

Project:

Bexhill - Hastings Link Road

Record of Borehole

BH194

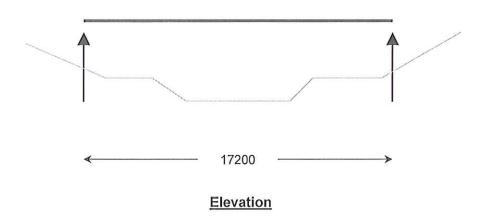
Client: East Sussex County Council

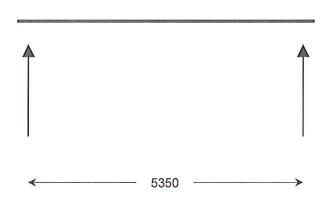
SAMI	PLES 8	& In situ TESTS	Ļ				STRATA
Depth	Type/ No.	SPT/U4 (Blows)	Water	Reduced Level (mOD)	Legend	Depth (Thick- ness)	DESCRIPTION SEE
Depth 0.30 0.50 1.00 1.50 2.00 2.50 3.00 4.00 4.50	D1 D2			3.38	<u> </u>	0.30	Firm brown slightly sandy slightly clayey SILT. Sand is fine to medium. Fine rootlets. (TOPSOIL) Firm light grey and brown very slightly gravelly slightly sandy slightly clayey silt. Sand is fine to coarse. Gravel is weak brown-light grey subangular to sub-rounded fine to medium siltstone and flint. Occasional fine to medium red brick, fine charcoal and chalk fragments. (MADE GROUND)
1.00	D3 U4	(70 - 450mm)		2.68	×××, × × × , × × × ,	_1.00	Firm to stiff laminated light grey and orange-brown stained orange-brown SILT. (Zone IV to II) (ASHDOWN BEDS)
2.00	U6	(90 - 450mm)		-	× × × × × × × × × × × × × × × × × × ×	-	
2.50	D7 SPT8	N=43 (2/6/9/10/12/12)			* * * ; * * * ; * * * ; * * * ;	-	At 2.5m bgl stiff to very stiff fissured light grey and orange-brown SILT. Orange-brown staining along fissure surfaces.
3.50	U9 D10	(120 - 250mm)		-	~ × × × × × × × × × × × × × × × × × × ×	-	At 3.5m bgl very stiff very closely fissured yellow-orange-brown SILT. Orange-brown staining along fissure surfaces.
4,00	U11	(120 - 350mm)		-	× × × × × × × × × × × × × × × × × × ×	-	orango orani didining diong nosore danaces.
	D12 SPT13	N=72 (8/10/15/15/20/22)		-	× × × × × × × × × × × × × × × × × × ×		At 4.5m bgl extremely closely to very closely fissured light grey and orange-brown SILT. Orange-brown staining along fissure surfaces.
5.50	D15	(120 - 450mm) (75 - 450mm)		-	× × × × × × × × × × × × × × × × × × ×	(9.00)	At 5.5m bgl very stiff fissured dark grey stained dark brown SILT, Dark brown staining along fissure surfaces.
6.50	D17 SPT18	N=38 (3/5/7/9/10/12)		-	× × × × × × × × × × × × × × × × × × ×	-	At 6.5m bgl very stiff thickly laminated dark grey slightly clayey SILT,
7.00 7.25	U19 D20	(120 - 150mm)		-	<]	From 7.25m bgl becoming very stiff possibly fissured thickly to thinly laminated grey SILT with frequent light grey very slightly sandy silt partings. Sand is fine.
Date	Time	g Progress and V Hole Cas'g Cas'g Depth Depth Dia		er Rose		Sealed	Chiselling Water Added From To Time (hh:mm) From To To Time (hh:mm) From To Draft Hand dug inspection pit to 1.2m bg Zone of weathering interpreted from Spinks et al. 1993: (V); (IV); (III); (II); (II); (II); (III); (III); (III); (III); (III); (III); (III); (III); (III); (IIII); (IIII); (IIII); (IIII); (IIIII); (IIIIIIIIII
Logged by: JB Checked by CAB Status: Draft		Cable Percussion Rig - Dando 2000					Ground Level: Date: Scale: 3.68 06/03/2009 Start 07.2 N 09/03/2009 End Sheet 1 of 2

Contra	ct No: ∠	1932	5727											UR	S
Project	t: E	3exhi	II - Ha	asting	js l	ink R	oad							Record of Borel	nole
	E	East :	Susse	x Co	un	ty Cou	ncil							BH194	4
SAMI			situ TE		, 5					STR	ATA				
Depth	Type/ No.		SPT/U/ (Blows		Water	Reduced Level (mOD)	Legend	Depth (Thick- ness)			DESCF	RIPTION			ment/
8.00	SPT21		N=43 (3/5/7/9/10/	17)			× × × × × × × × × × × × × × × × × × ×	-							
8.50	D22						\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \								
9.00	U23 B24		(150 - Omr	n)			- × · ; × × ; × × ;								
9.50	SPT25		N=40 (5/6/7/8/12/	13)			× × × × × × × × × × × × × × × × × × ×								
10.00	D26					-6.32	Û×Û)	10.00	Slightly moist		d of Boreh	ole at 10.0	0m		
Depth 8.00 8.50 9.00 9.50 10.00	1	ng Pro	gress Cas'g			er Obseter Ros				selling	-	Added		GENERAL REMARKS	
Date 06/03/09 09/03/09	Time 17,00 10.30	Depth 8.50 10.00	Depth 4.00 7.00	Dia 150 150	Dep 4.5	oth to	(min		From	(hh:mn	1) FIOR	То	Draft Hand du Zone of v from Spir (III); (II);(I	g Inspection pit to 1.2 veathering interprete kks et al. 1993: (V); ().	2m bgl ed from (IV);
Checked b	4	Contr	Percuss actor:			ando 200		7	n: 81.9 E 07.2 N	Ground 3.68 mAOD	11.000	Date: 06/03/200 09/03/200		Scale: 1:40.0	
Draft		South	ern Test	ing Lat	orat	ories Ltd		1,100	UI,LIN					Sheet 2 of 2	

Appendix D Idealised Structure

DIAGRAM OF IDEALISED STRUCTURE TO BE USED IN ANALYSIS





Sectional View

Appendix E Departures from Standards

Departure # Hydrophobic Pore Lining Impregnant

BD 43/03 specifies various requirements for the impregnation of highway structures with hydrophobic pore lining impregnant. Following the completion of research into the long term effectiveness of hydrophobic pore lining impregnants on concrete highway structures, the Highways Agency is temporarily suspending requirements to apply all such impregnants as set out in BD43/03.

This suspension is detailed in CHE Memorandum 227/08 - The Impregnation Of Reinforced and Prestressed Concrete Highway Structures Using Hydrophobic Pore Lining Impregnants

This Departure seeks to apply this suspension to structures on the BHLR – i.e. the impregnant will not be applied.

This will not preclude the opportunity to apply impregnant in the future should this be required.

Departure # Longitudinal Loading on Greenway Structures

This Departure seeks approval:

- to delete the BD37/01 Clauses 6.10 and 6.11 requirements for longitudinal load for traction and braking and accidental load due to skidding.
- to apply instead a single alternative nominal longitudinal load of 150 kN. This load will be applied as described in BD37/01 Clause 6.10

The BD37 requirements for these loadings are based on significantly higher traffic loads and speeds than the Greenway structures will be subjected to. The 150 kN proposed is based on the maximum horizontal load that can be generated by the 24 tonne design vehicle reacting on the structure with a coefficient of friction of 0.6 between vehicle and deck. We consider that this approach is moderately conservative and appropriate to the structures concerned.

Departure # Combined Waterproofing and Surfacing on Steel Bridge Decks

This departure seeks approval for use of proprietary product for waterproofing and anti slip treatment of the steel bridge decks. This is required as waterproofing and corrosion protection of steel bridge decks is not covered in Clause 1802 - surface preparation and protection against corrosion – Specification, Structures SHW, MCHW Vol 1, Series 1800 – Structural Steelwork.

It is proposed to use a resin based system with a slip resistant aggregate dressing e.g. Bimagrip or CICOL.

Use of such products is typical on steel footbridges and link span bridges and is recommended for use on equestrian bridges by the British Horse Society.

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